

#### Introduction



#### Name

# Affiliation

What is YOUR biggest challenge in stormwater control measure accounting/planning?

#### Opti-Tool Outreach Workshop Part 1

Research Based Stormwater System Accounting

New England Environmental Finance Center

November, 2017







## Agenda for the Day



#### Introduction

**Unit Operations & Processes (UOPs)** 

#### **Review of BMPs**

**Review of BMP Worksheets and Cross-walks** 

#### Costs

**Effective SWM Case Study** 

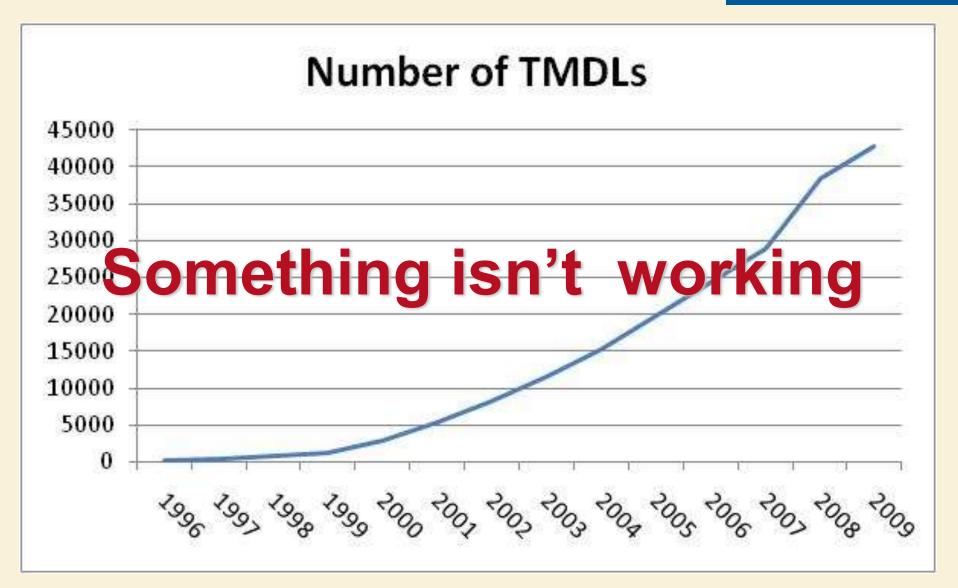
**Site Design Assessment** 

**Group Debrief & Discussion** 



# We're Here

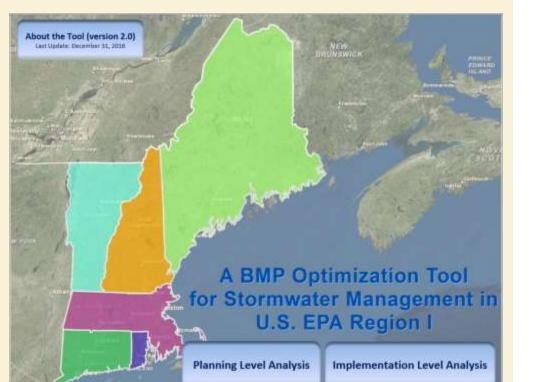






#### What is the Opti-Tool





MA MS4 General Permit

United States Environmental Protection Agency (EPA) National Pollutant Discharge Elimination System (NPDES)

#### GENERAL PERMITS FOR STORMWATER DISCHARGES FROM SMALL MUNICIPAL SEPARATE STORM SEWER SYSTEMS IN MASSACHUSETTS

AUTHORIZATION TO DISCHARGE UNDER THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM

In compliance with the provisions of the Clean Water Act (CWA), as amended (33 U.S.C. §1251 et seq.), and the Mausachusetts Clean Waters Act, as amended (M.G.L. Chap.21 §§ 26-53), any operator of a small municipal separate storm sewer system whose system:

- · Is located in the areas described in part 1.1;
- · Is eligible for coverage under part 1.2 and part 1.9; and
- Submits a complete and accurate Notice of Intent in accordance with part 1.7 of this permit and EPA issues a written authorization

is authorized to discharge in accordance with the conditions and the requirements set forth herein.

The following appendices are also included as part of these permits:

- Appendix A Definitions, Abbreviations, and Acronyms;
- Appendix B Standard permit conditions applicable to all authorized discharges;
- Appendix C Endangered Species Act Eligibility Guidance:
- Appendix D National Historic Preservation Act Eligibility Guidance;
- Appendix E Information required for the Notice of Intent (NOI);
- Appendix F Requirements for MA Small MS4s Subject to Approved TMDLs;
- Appendix G Impaired Waters Monitoring Parameter Requirements;
- Appendix H Requirements related to discharges to certain water quality limited waterbodies;

These permits become effective on July 1, 2017.

These permits and the authorization to discharge expire at midnight, June 30, 2022.

Signed this 11th day of April 2016

Vita

Ken Moraff, Director Office of Ecosystem Protection United States Environmental Protection Agency 5 Post Office Square – Suite 100 Boston, Masnachmerts 02109-3912

Signed this 4" day of April 2014

Boglas E. Fine Assistant Commissioner for Water Resources Department of Environmental Protection One Winter Street Boston, Massachusetts 02108



# Materials We Will Use Today



#### Infiltration Trench Factabert

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## Agenda for the Day



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**Unit Operations & Processes (UOPs)** 

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**Review of BMP Worksheets and Cross-walks** 

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**Site Design Assessment** 

**Group Debrief & Discussion** 



mwater System Classification Be Challenging



## Because we don't always speak the same language

Swale Permeable Interlocking Concrete Pavers Permeable Interlocking Concrete Pavers Subsurfacementeriowater Cuatrity meters Permeable Interlocking Concrete Pavers Downstream Defender Bio-Swale lized Basins Permeable Interleckfre Storm orm Trooper Permeable Intern ekenkavenscrete Paverser Pierretentificheterentificher Stenner eter Bisters Filtera Sand Eil eable Interlocking Concrete Pavers Gravel Wetla eable Interiocking Concrete Bayers Wetland **J**cted Wetland



#### Imagine the Ultimate System...







## **Now Consider Bioretention**







# No Need to Reinvent this Wheel



# **Use Unit Operations & Processes (UOPs)**

- Physical Operations
- Biological Processes
- Chemical Processes
- Hydrologic Operations





# **Physical UOPs**



# SedimentationEnhanced SedimentationFiltrationScreening



# **Biological UOPs**



#### **Vegetative Process**

#### **Microbial Process**





# **Chemical UOPs**



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## Hydrologic UOPs



#### **Flow Alteration**

#### **Volume Reduction**





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## **Opti-Tool**



ВМР Туре	Design Storage Capacity (ft <sup>3</sup> )	в	MP Cost (\$)	Treated Impervious Area (ac)	O&M (hr/yr)	Load Reduction (Ibs)	Treated Runoff Depth (in)
Biofiltration with ISR	-	\$	-	-	-	-	-
Bioretention	-	\$	-	-	-	-	-
Dry Pond	-	\$	-	-	-	-	-
Grass Swale*	-	\$	-	-	-	-	-
Gravel Wetland	-	\$	-	-	-	-	-
Infiltration Basin	-	\$	-	-	-	-	-
Infiltration Chambers*	-	\$	-	-	-	-	-
Infiltration Trench	-	\$	-	-	-	-	-
Porous Pavement*	-	\$	-	-	-	-	-
Sand Filter	-	\$	-	-	-	-	-
Wet Pond	-	\$	-	-	-	-	-
New York, Children and State 1997	the second se						

Note:Only fill in the yellow highlighted cells.

\* Place holder for future option (not implemented)

https://www.epa.gov/tmdl/opti-tool-epa-region-1s-stormwater-management-optimization-tool



# **SCMs Currently Covered in Opti**



# **Covered in Opti (8)**

- Biofiltration with ISR
- Biofiltration
- Dry Pond
- Gravel Wetland
- Infiltration Basin
- Infiltration Trench
- Sand Filter
- Wet Pond

# Covered in MS4 (2)

- Pemeable (Porous) Pavement
- Grass Swale



#### **Some Definitions**



# Design Storage Volume (DSV), aka Design Storage Capacity

L = length

W = width

**D** = depth at design capacity before bypass

#### n = porosity/void space of fill material

- soil media (bio-retention soil mix, engineered soil mix, etc.) = 0.2
- peastone (<sup>3</sup>/<sub>8</sub>" washed) = 0.3
- reservoir stone (¾" washed) = 0.4

#### A = average surface area for calculating volume



# **DSV Example**



# Infiltration Basin/Surface Infiltration for raingarden or bio-retention with no underdrains

- DSV = Ponding water storage volume and void space volumes of soil filter media and stone layers, if applicable.
- DSV =  $(A_{pond} \times D_{pond}) + (A_{soil} \times D_{soil} \times n_{soil}) + (A_{stone} \times D_{stone} \times n_{stone})$



# **DSV Example**



# Infiltration Basin/Surface Infiltration for raingarden or bio-retention with no underdrains

- DSV =  $(A_{pond} \times D_{pond}) + (A_{soil} \times D_{soil} \times n_{soil}) + (A_{stone} \times D_{stone} \times n_{stone})$
- A = 100 sf Ponded Depth = 1' Soil Depth = 2' Stone Depth = 1.5'
- $DSV = (100 \times 1 \times 1) + (100 \times 2 \times 0.2) + (150 \times 1 \times 0.4)$ DSV = 200 cf



# **Treated Runoff Depth**



# = DSV/WQV x 12

Drainage Area	1 acre (43,560 sf)
Impervious Area	0.6 acres (26,136 sf)
WQV = Area sf x $1/12$	2,000 cf
DSV	200 cf
Treated Runoff Depth	0.1 inches



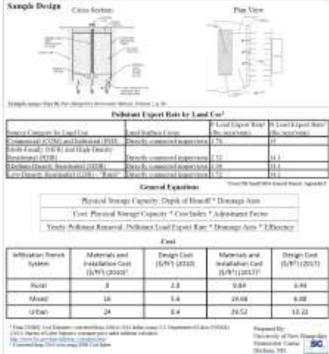
#### **Review of Worksheets**

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#### Infittation Trench Fectsheet

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# **Fill in the Blanks**



ВМР Туре	DSV (cf)	Treated IC acres (sf)	Treated Runoff Depth	% Load Reduct TSS TP		ction TN
Bioretention/ Infiltration Basin	200	1 (26,136)	1.2			

#### HSG A = 8.24 in/hr



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#### Life Cycle Costs Including Maintenance



вмр	Area of IC treated	WQv (cf)	BMP Area ft3	Annual Ave Maintenance \$	Annual Maintenance hours	Capital Cost per acre of IC	2010 Adj Capital Cost of system *	Added design contingency of 35%			Capital Costs per BMP storage volume	Capital Costs/sf of IC	Amoritized Life Cylce Costs ***
						treated	of system *	01 35%		(WQv)	(cf)		
Vegetated Swale	1.00	3,630	5,400	\$822.50	9.5	\$11,200.00	\$12,928.68	\$17,453.72	\$3.23	\$4.81	NA	\$0.26	\$33,903.72
Retention Pond	1.00	3,630	12,880	\$3,060.00	28.0	\$13,700.00	\$15,814.54	\$21,349.63	\$1.66	\$5.88	\$5.88	\$0.31	\$82,549.63
Detention Pond	1.00	3,630	12,880	\$2,380.00	24.0	\$13,700.00	\$15,814.54	\$21,349.63	\$1.66	\$5.88	\$5.88	\$0.31	\$68,949.63
Chamber System	1.00	3,630	434	Not assessed	Not assessed	\$34,000.00	\$34,434.75	\$46,486.91	\$107.13	\$12.81	\$107.13	\$0.78	Not assessed
Sand Filter	1.00	3,630	640	\$2,807.50	28.5	\$12,417.00	\$14,333.52	\$19,350.25	\$30.23	\$5.33	\$15.51	\$0.29	\$75,500.25
Gravel Wetland	1.00	3,630	1,920	\$2,138.33	21.7	\$22,300.00	\$25,741.92	\$34,751.59	\$18.10	\$9.57	\$7.59	\$0.51	\$77,518.26
Bioretention	1.00	3,630	4,326	\$1,890.00	20.7	\$20,000.00	\$23,086.92	\$31,167.34	\$7.21	\$8.59	\$13.37	\$0.46	\$68,967.34
Enhanced Bio	0.39	935	373	\$1,890.00	21.0	\$29,000.00	\$29,000.00	\$39, 150.00	\$105.09	\$41.86	\$105.09	\$0.67	\$68,967.34
Porous Asphalt	1.00	3,630	32,670	\$1,080.00	6.0	\$21,780.00	\$22,058.49	\$29,778.96	\$0.91	\$8.20	\$4.60	\$0.50	\$51,378.96
Pervious Concrete**	1.00	3,630	32,670	\$1,080.00	6.0	\$74,052.00	\$74,998.88	\$101,248.49	\$3.10	\$27.89	\$15.63	\$1.70	\$122,848.49
Permiable Interlocking Concrete Pavement **	1.00	3,630	32,670	\$1,080.00	6.0	\$74,052.00	\$74,998.88	\$101,248.49	\$3.10	\$27.89	\$15.63	\$1.70	\$122,848.49

note all costs were converted from 2004 dollars to 2010 dollars with the exception of the permeable pavements which were converted from 2008 dollars to 2010 dollars

\* See reference information from USDOL

\*\* PA cost estimates were calculated as the difference between PA installations and a typcical dense mix pavement equivalent. PC and PICP costs were developed using the same methodology and compared to typical DMA, not typical concrete or paver pavements.

\*\*\* Life cycle costs were calculated based on 2010 capital costs and amoritized annual maintenance costs over an expected useful life of 20 years

#### https://www3.epa.gov/region1/npdes/stormwater/ma/green-infrastructure-stormwater-bmp-costestimation.pdf



# Design, Capital, and Construction Costs



Infiltration Trench System	Materials and Installation Cost (\$/ft <sup>3</sup> ) (2010) <sup>2</sup>	Design Cost (\$/ft <sup>3</sup> ) (2010)	Materials and Installation Cost (\$/ft <sup>3</sup> ) (2017) <sup>3</sup>	Design Cost (\$/ft³) (2017)
Rural	4	1.88	4.92	1.72
Mixed	8	3.76	9.84	3.44
Urban	12	5.64	14.76	5.16



# **Region 1 GI Cost Estimates**



BMP (From Opti-Tool)	Cost (\$/ft <sup>3</sup> ) <sup>1</sup>	Cost (\$/ft³) – 2016 dollars <sup>6</sup>	
Bioretention (Includes rain garden)	13.37 <sup>2,4</sup>	15.46	
Dry Pond or detention basin	5.88 <sup>2,4</sup>	6.80	
Enhanced Bioretention (aka-Bio-filtration Practice)	13.5 <sup>2,3</sup>	15.61	
Infiltration Basin (or other Surface Infiltration Practice)	5.4 <sup>2,3</sup>	6.24	
Infiltration Trench	10.8 <sup>2,3</sup>	12.49	
Porous Pavement - Porous Asphalt Pavement	4.60 <sup>2,4</sup>	5.32	
Porous Pavement - Pervious Concrete	15.63 <sup>2,4</sup>	18.07	
Sand Filter	15.51 <sup>2,4</sup>	17.94	
Gravel Wetland System (aka-subsurface gravel wetland)	7.59 <sup>2,4</sup>	8.78	
Wet Pond or wet detention basin	5.88 <sup>2,4</sup>	6.80	
Subsurface Infiltration/Detention System (aka- Infiltration Chamber)	<b>54.5</b> 4⁵	67.85	

<sup>1</sup> Footnote: Includes 35% add on for design engineering and contingencies



# **Fill in the Blanks**



ВМР Туре	DSV (cf)	Treated IC acres (sf)		Treated Runoff	% Load Reduction		
				Depth	TSS	TP	TN
Bioretention/ Infiltration Basin	200	1 (2	6,136)	1.2	99	99	99
ВМР Туре			BMP Cost (\$/yr) O&M Costs (hrs/y		rs/yr)		
Bioretention/Infiltration Basin					12.42		

# Assume Urban Environment 14.76 + 5.16 = 19.92 x 200 = \$3,984



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Hand out case description of Daisy Field project: case should be designed to prompt/spark thinking about alternatives using GI/BMP approaches; Hold Discussion. 25 minutes





# Daisy Field

- 47.4 Acre Ultraurban environment at 65% IC
- Not a lot of space to put Bmps
  - 62% TP reduction requirement as the discharge is to the Charles River



# Watershed Characteristics



	Daisy Field
Ρ	1
A	70.47
I	0.65
Rv	0.635
WQV (acre-in)	1.88
WQV (ft <sup>3</sup> )	81 <i>,</i> 936.36
S	1.11
Q (in)	0.320325
CN	90
la	0.22
la/P	0.22
TC (hr)	0.12



# What would you do?







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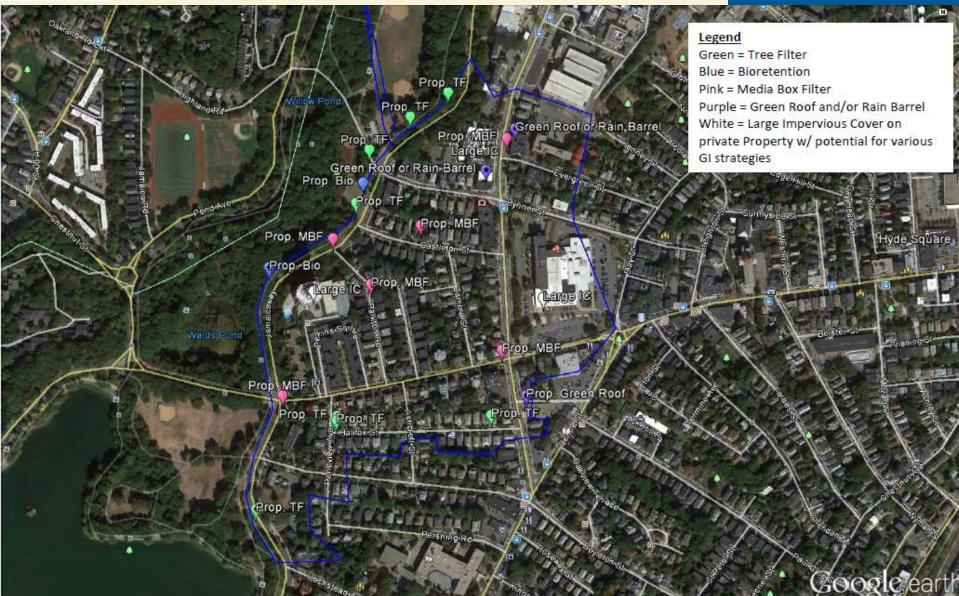
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#### **Other Considerations**





	Location	Best Management Practice	Rationale	SC
New England Environmental Finance Center	Along west side of Jamaica Way	Tree Filters (5)	Existing storm drain line and several catch basins. There are also some recently planted trees that could be transplanted into tree filter units. These would provide some treatment through filtration, infiltration, rhizosphere, and flow attenuation. Media amendments would be added to remove phosphorus.	BRIVERSITY OF NEW STORMWATER
	Halifax St and Pondview Ave	Tree Filters (2)	There is an existing storm drain line and catch basins on each side of the Halifax St. The existing street scape includes trees.	
	Halifax St near S. Huntington St	Tree Filter	There is an existing storm drain line and a catch basin that could be converted to a tree filter.	
	<ul> <li>Parkwood Terrace and Jamaica Way</li> <li>Perkins St and Jamaica Way</li> </ul>	Media Box Filters	Existing storm drain line and catch basin. Large vegetated strip between road and sidewalk that is likely in City right-of-way. Large trees are in this proximity so an MBF would be a better fit.	
	<ul> <li>North end of Parkton Rd</li> <li>Perkins St and S. Huntington</li> <li>Castleton St</li> <li>Evergreen St and S. Huntington</li> </ul>	Media Box Filters	Existing storm drain line and catch basins on each side of Perkins St. No vegetated area but an MBF might fit under the road depending on other utilities.	
	Just south of Highland Rd and Jamaica Way intersection	Bioretention	Existing storm drain line and room for a bioretention cell that could treat surface runoff and then be piped into existing line.	
	West of Jamaica Way near Wards Pond	Bioretention	Plenty of area for a bioretention cell but some trees may need to be removed. Effluent could discharge cool treated water to Wards Pond.	
	<ul> <li>El Mundo Newspaper</li> <li>Intersection of Bynner St and South Huntington</li> <li>Intersection of Evergreen St and South Huntington</li> </ul>	Green Roof and Rain Barrel installations	These are flat roof buildings with concrete block or brick exteriors, which may be strong enough for green roof systems. There are several other buildings in the drainage area that could be identified if this is a viable strategy.	
	<ul> <li>MSPCA, Animal Care &amp; Adoption Center</li> <li>Perkins Square – Sagamore Advisors</li> <li>Mt. Pleasant Home</li> </ul>	Tree Filters, Bioretention(s), media box filters	Three of the largest, privately owned impervious cover areas in the watershed. There are media strips, large parking areas, and large roof tops that could all be managed using various GI strategies.	



CENTER







"Hi, Tim and I were just chatting about siting systems. Is there any reason why we could not put a system where the orange oval is in the pic below?"

"I'm going to say almost definitively no. It's private property and we have no way to get those property owners to work with us. Additionally, my understanding is that we want a visible area for public education (a park in this tributary area). "





We need to avoid the ball fields for now. In the future, Parks may choose to redo the fields and we will propose an Underground Gravel Filter at that time.

### What are we trying to treat?

- We are trying to treat the first 1" of water across the whole catchment
- If that cannot be done, then aim for the 62% reduction of phosphorus

### Who owns what?

- a. Parks owns the pipe that runs under Daisy Field, starting from 18GMH252 and continuing to the outfall
- i. However, BWSC owns 18GMH252
- b. Parks also owns the outfall into Leverett Pond





In the "TOPOGRAPHY" layer there are several sub-layers that seem to be paired for identical points on the map. They consistently differ by 6.5'. Which surface elevation layer is correct or relative to the Pipe Invert Elevation layers? For example:

Sub-Layer Name	Elevation	Elevation	Elevation	General Location of Point
Topo DETBCB Elevation	29.0	34.8	28.3	East of Jamaica Way / Willow Pond Rd
				intersection.
Topo DET Elevation	22.5	28.3	21.8	Same as above
Difference	6.5	6.5	6.5	
Topo INDBCB Elevation	26.5			On contour line southeast of Jamaica Way / Willow Pond Rd. int.
Topo INDD Elevation	20			Same as above
Difference	6.5			
Topo BCB Elevation	33.0	31.8	34.2	In vicinity of Jamaica Way / Willow Pond Rd. intersection
TOP GEN Elevation	26.5	25.3	27.7	Same as above
Difference	6.5	6.5	6.5	





Here are the updated 90% Concept Designs for Daisy Field Stormwater Infrastructure

Bio-2 is probably too close to the heritage oaks for comfort, but we can leave it there for now.

Bio-4 is located in the primary walking path from the parking lot to the softball diamonds and is unlikely to be acceptable to Parks.

The current location of the Subsurface Gravel Wetland is not an option and will need to be moved.





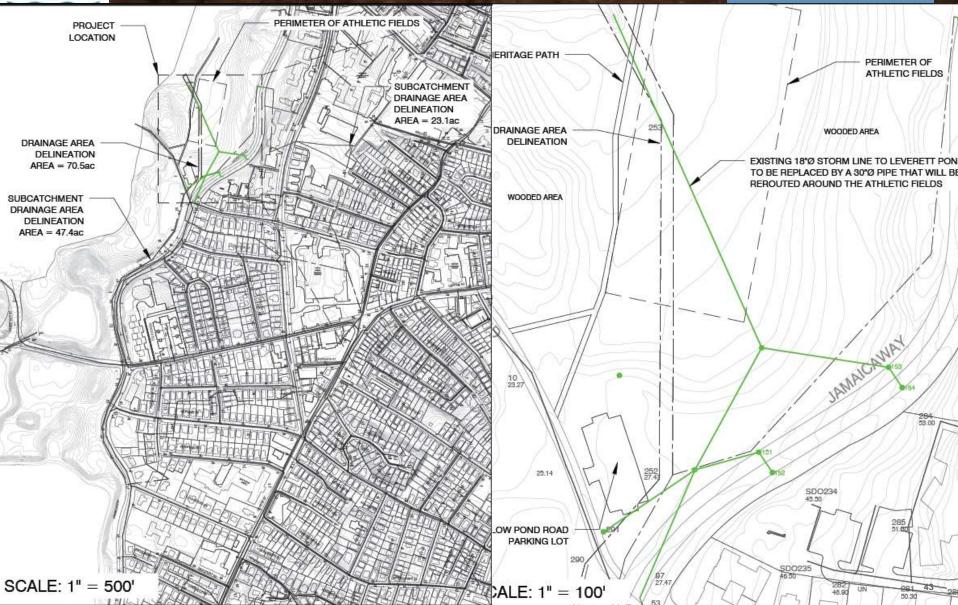
Well, lo and behold, after our initial meeting and site walk with Parks, if some scheduling and budgeting concerns can be addressed with Parks' long planned Daisy Field renovation, we might be able to do something under the athletic fields (assuming that grass and not artificial turf can be used).

We are still waiting for a letter from Parks with the official feedback, but based on what I heard during the meeting and the walk, we need to start putting together final pollutant removal and cost estimates for a feature under the fields



## What we did...

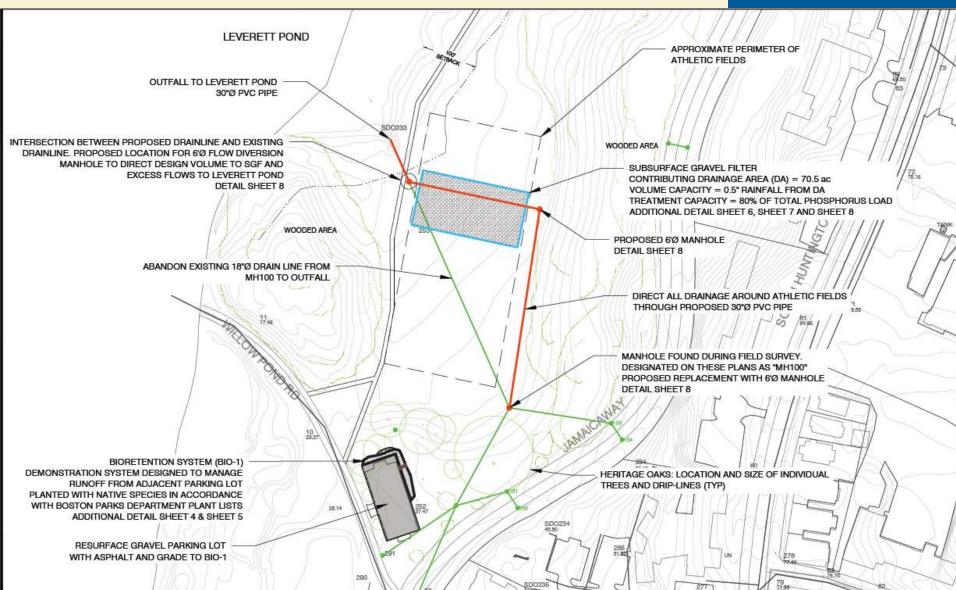






### **Final Design**

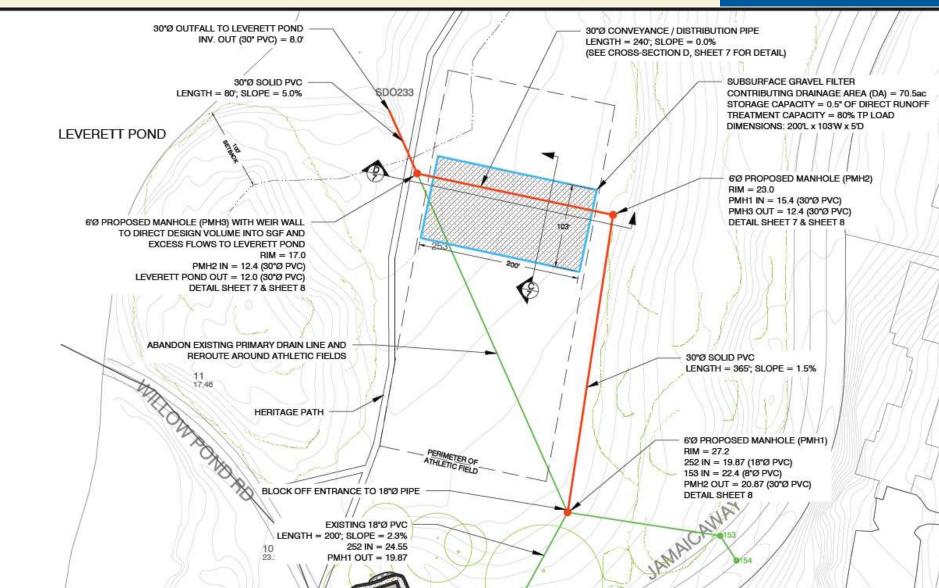


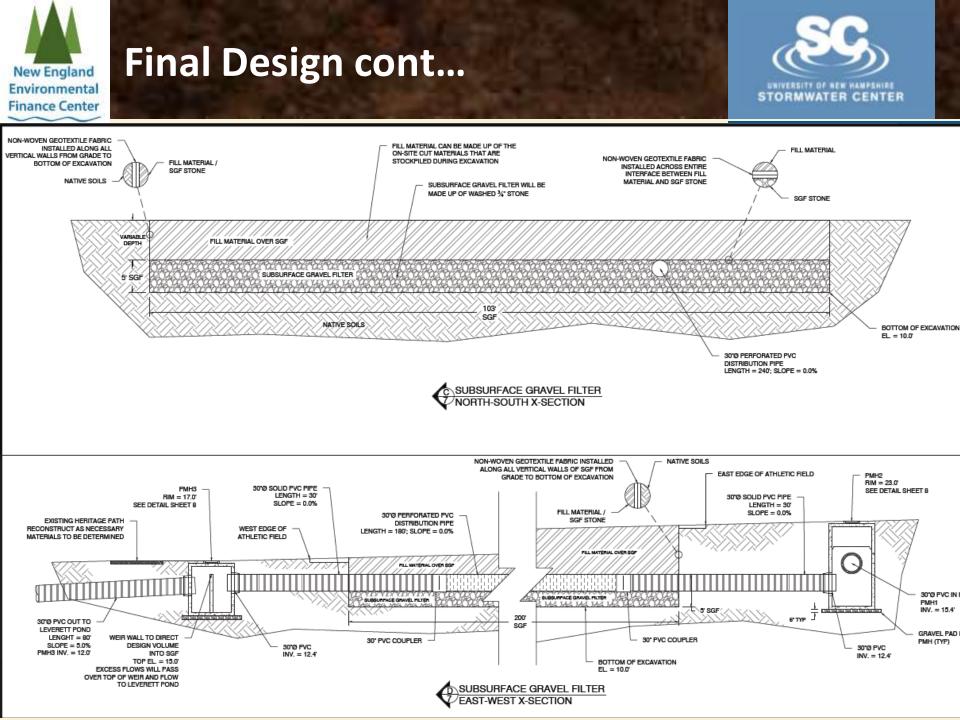




### **Final Design cont...**









**BMP Capacity: Depth of Runoff** 

**Treated from Impervious Area** 

(inches) Runoff Volume Reduction

**Cumulative Phosphorus Load** 

Reduction

Infiltration Trench (IR = 1.02 in/hr) BMP Performance Table: Long-Term Phosphorus Load Reduction

0.2

44.6%

47%

0.1

26.3%

27%

0.4

68.2%

73%

0.6

81.0%

86%

0.8

88.0%

92%

1.0

92.1%

96%

96.5%

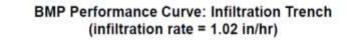
99%

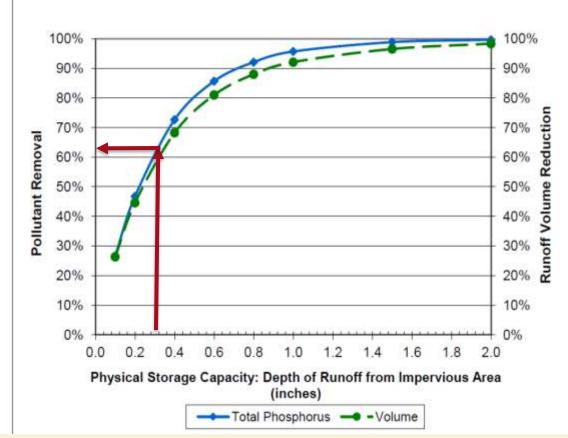
98.

10











Results



Stormwater Management Design - 70.5 acre Ultra-Urban Drainage Area									
Sizing Comparison of Capital Costs and Relative Phosphorus Load Removal Efficiency									
Best Management Practice Size	Depth of Runoff Treated from Impervious Area (in)	*Storage Volume Cost (\$/ft <sup>3</sup> )	**Total Phosphorus Removal Efficiency (%)						
Subsurface Gravel Filter - Minimum Size	0.35	\$1,016,912	62%						
Subsurface Gravel Filter - Moderate Size	0.5	\$1,452,732	80%						
Subsurface Gravel Filter - Full Size	1.0	\$2,905,463	96%						

\*Storage Volume Cost estimates provided by EPA-Region 1 for Opti-Tool methodology, 2015-Draft \*\*Total Phosphorus %RE based on Appendix F Massachusetts MS4 Permit



## Agenda for the Day



#### Introduction

**Unit Operations & Processes (UOPs)** 

### **Review of BMPs**

**Review of BMP Worksheets and Cross-walks** 

#### Costs

Effective SWM Case Study

**Site Design Assessment** 

**Group Debrief & Discussion** 



### **Next Steps**



Group discussion of best use of UNHSC data sheets, who could benefit from workshop 2 on Opti-Tool and automation/optimization approach.

Input to further design of Workshop 1 as well as prospectively for Workshop 2.



# **Questions?**

